

▪ **Lay out and concrete dimension**

$$R = 11.5 \text{ m}$$

$$\text{hole radiuce} = 3 \text{ m}$$

$$t = 25 \text{ cm}$$

▪ **Load**

$$L.L = 200 \text{ kg/m}^2$$

$$D.L = .25 * 2.5 = .625 \text{ t/m}^2$$

▪ **Straining action**

Load type	Case of loading	Radian moment		Tangential moment	
		Max	Min	Max	Min
All load	Ultimate	3.98	-7.2	1.08	-2.6
All load	Working	2.7	-5	.76	-1.8

Design Radian moment

Section 1

$$M_w = 2.7 \text{ m. t}$$

$$b = 100 \text{ cm}$$

$$t = 25 \text{ cm}$$

Water section

1- Working stage

$$F_{Ct-M} = \frac{6 * M_w}{b * t^2}$$

$$F_{Ct-M} = \frac{2.7 * 10 * 6 * 10^6}{1000 * 250^2} = 2.592 \text{ Mpa}$$

$$t_v = t * \left[ 1 \mp \frac{F_{Ct-N}}{F_{Ct-M}} \right]$$

$$t_v = 250 * \left[ 1 + \frac{0}{1.67} \right] = 250 \text{ mm} \quad \rightarrow \quad \eta = 1.32$$

$$F_{ctr} = .6 * \sqrt{F_{cu}}$$

$$F_{ctr} = .6 * \sqrt{30} = 3.286 \text{ Mpa}$$

$$\frac{3.286}{1.32} = 2.48 < F_{ct-M} \quad \text{section is unsafe}$$

يجب زيادة تخانة البلاطة لضمان عدم حدوث شروخ فيها

Load type	Case of loading	Radian moment		Tangential moment	
		Max	Min	Max	Min
All load	Ultimate	4.4	-7.74	1.2	-2.98
All load	Working	3	-5.44	.86	-2

$$t = \sqrt{\frac{M_w * 10^6}{.3 * B}}$$

$$t = \sqrt{\frac{2.6 * 10 * 10^6}{.3 * 1000}} = 294.390 \text{ mm} \cong 280 \text{ mm} \xrightarrow{\div 10} 28 \text{ cm}$$

**Check stress**

$$F_{ct-M} = \frac{6 * M_w}{b * t^2}$$

$$F_{ct-M} = \frac{3 * 10 * 6 * 10^6}{1000 * 280^2} = 1.98 \text{ Mpa}$$

$$t_v = t * \left[ 1 \mp \frac{F_{ct-N}}{F_{ct-M}} \right]$$

$$t_v = 280 * \left[ 1 + \frac{0}{1.67} \right] = 280 \text{ mm} \quad \rightarrow \eta = 1.42$$

$$F_{ctr} = .6 * \sqrt{F_{cu}}$$

$$F_{ctr} = .6 * \sqrt{30} = 3.286 \text{ Mpa}$$

$$\frac{3.286}{1.42} = 2.31 > F_{ct-M} \quad \text{section is safe}$$

Ultimate stage

**Concrete cover**

From table 4-13  $d' = 25 \text{ mm}$

$$M_u = 4.4 \text{ m.t}$$

$$d = c_1 \sqrt{\frac{M_u}{F_{cu} * b}}$$

$$28 - 2.5 = c_1 * \sqrt{\frac{4.4 * 10^5}{300 * 100}} \quad \therefore c_1 = 6.65 \quad \rightarrow J = 0.826$$

$$\beta_{crack} = .75 \quad \text{form coad table 4 - 15}$$

$$A_s = \frac{M_u}{F_y * J * d * \beta_{crack}}$$

$$A_s = \frac{4.4 * 10^5}{3600 * .826 * 25.5 * .75} \quad A_s = 7.7369 \text{ cm}^2$$

Check of minimum area of steel

	$A_{s \min} = 1.3A_s$
$\max$ of	$A_{s \min} = \frac{11}{3600} * b * d$
	$A_{s \min} = \frac{0.15}{100} * b * d$

$$A_{s \min} = 3.83 \text{ cm}^2$$

$$A_{s \min} < A_s \quad \text{used } A_s = 7.7369 \text{ cm}^2 \rightarrow 7\emptyset 12 / m$$

Design tangential moment

$$M_u = 1.2 \text{ t.m}$$

Water section

$$1.2 \ll 3 \text{ section is safe}$$

$d = c_1 \sqrt{\frac{M_u}{F_{cu} * b}}$
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$$28 - 2.5 = c_1 * \sqrt{\frac{1.2 * 10^5}{300 * 100}} \quad \therefore c_1 = 12.75 \rightarrow J = 0.826$$

$$\beta_{crack} = .85 \quad \text{form coad table 4 - 15}$$

$A_s = \frac{M_u}{F_y * J * d * \beta_{crack}}$
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$$A_s = \frac{1.2 * 10^5}{3600 * .826 * 25.5 * .85} \quad A_s = 1.861 \text{ cm}^2$$

Check of minimum area of steel

	$A_{s \min} = 1.3A_s$
$\max of$	$A_{s \min} = \frac{11}{3600} * b * d$
	$A_{s \min} = \frac{0.15}{100} * b * d$

$$A_{s \min} = 3.83 \text{ cm}^2$$

$$A_{s \min} > A_s \quad \text{used } A_{s \min} = 3.83 \text{ cm}^2 \rightarrow 5\emptyset 10 / m$$

تخاتو البلاطة اكثر من 16 سم طبقا للكوود المصري يجب إضافة شبكة علوية تتثل نسبة 20 % من الحديد الرئيسي في كل اتجاه ولا تقل عن في كل اتجاه

used 5Ø8 radin and tangentaial

الغي كل قيم المومنت الي مش ليها أي 30 لازمة