

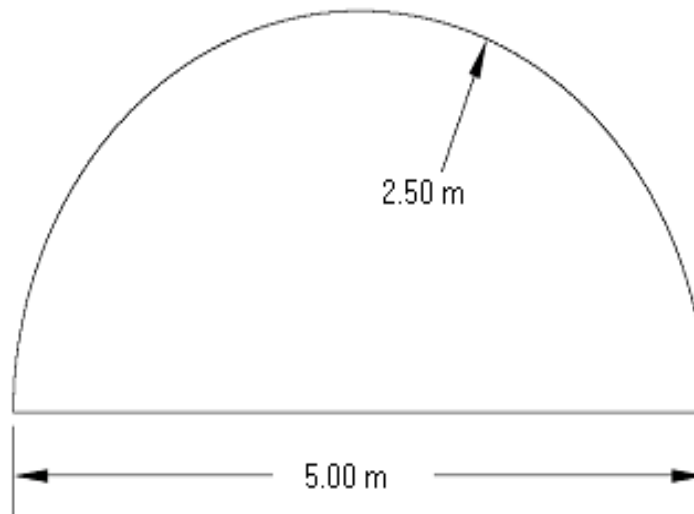
2 - Semi-circle dome .

Geometry:

From the drawing given by the architectural engineer we get:

Take thickness $t=10\text{cm}$

$R = 2.5 \text{ m}$



Calculation of Loads:

A - Dead load:

Selfweight= $0.10 \times 25 = 2.5 \text{ KN/m}^2$

plastering 2cm each side

Plastering= $0.04 \times 20 = 0.8 \text{ KN/m}^2$

Dead Load= $2.5 + 0.8 = 3.3 \text{ KN/m}^2$

B - Snow load:

According to the Jordanian code the snow load = 1 KN/m^2

Total load $W = 3.3 + 1 = 4.3 \text{ KN/m}^2$

C - Concentrated load due to the chandelier:

Assume the weight of the chandelier (P_c) = 2.5 KN

Analysis:

calculate the meridian and hoop stresses on the dome:

A - Meridian stresses: $N_\phi = -W \times R \times K_1 + \left(\frac{P_c}{R} \right) \times K_3$

B - Hoop stresses: $N_\theta = -W \times R \times K_2 - \left(\frac{P_c}{R} \right) \times K_3$

Where:

$$K_1 = \frac{1 - \cos\theta}{\sin^2\theta}, \quad K_2 = \frac{\cos^2\theta + \cos\theta - 1}{1 + \cos\theta}, \quad K_3 = \frac{\operatorname{cosec}^2\theta}{2\pi}$$

Calculation of K_1 , K_2 , K_3

Angle	K_1	K_2	K_3
0	0.500	0.500	---
10	0.505	0.480	5.300
20	0.516	0.425	1.370
30	0.537	0.330	0.640
40	0.566	0.200	0.380
50	0.608	0.034	0.270
60	0.667	-0.167	0.210
70	0.747	-0.402	0.180
80	0.838	-0.680	0.160
90	1.000	-1.000	0.160

Calculation of the meridian and hoop stresses:

Angle(°)	Meridian stresses N_ϕ (KN/m')	Hoop stresses N_θ (KN/m')
0	---	---
10	-0.129	-10.46
20	-4.177	-5.94
30	-5.133	-4.188
40	-5.71	-2.53
50	-6.27	-0.636
60	-6.96	1.59
70	-7.85	4.14
80	-8.85	7.15
90	-11.6	10.59

Maximum meridian stress $N_{\theta} = -11.6 \text{ KN/m}$ (compression stress)

Maximum hoop stress $N_{\phi} = 10.59 \text{ KN/m}$ (tensile stress) at 90°

$$N_{\phi} = \frac{-11.6}{t} = \frac{-11.6}{0.10} = -116.0 \text{ KN/m}^2 = -0.1043 \text{ N/mm}^2$$

$$N_{\theta} = \frac{10.59}{t} = \frac{10.59}{0.10} = 105.9 \text{ KN/m}^2 = 0.1059 \text{ N/mm}^2$$

Check for maximum tensile strength of concrete

$$T_c = 10\% \times 0.4 \times f'_c = 0.1 \times 0.4 \times 25 = 1 \text{ N/mm}^2 > 0.1059 \text{ N/mm}^2$$

Because the maximum tensile strength of concrete is greater than maximum hoop stress, only nominal reinforcement of 0.2% is provided.

Design of steel :

$$A_s = 0.2\% \times t \times 1\text{m} = 0.002 \times 100 \times 1000 = 200 \text{ mm}^2/\text{m}$$

Use $\phi 8$

$$\# \text{ of bars/m} = \frac{A_s}{\text{Area of } \phi 8 \text{ bar}} = \frac{200}{50} = 4 \text{ bars/m}$$

$$\text{Spacing} = \frac{1000\text{mm}}{4\text{bar}} = 250\text{mm}$$

Provide $\phi 8 @ 250\text{mm c/c}$ (hoop direction)

Provide $\phi 8 @ 250\text{mm c/c}$ (meridian direction)